



## Basic knowledge

## BUCKLING AND STABILITY

If slim and long components such as bars, beams and columns are subject to compressive stress owing to a force along the bar axis, these can end up in indifferent or unstable equilibrium positions. If the force  $F$  is less than the critical force  $F_K$ , also known as buckling force, the component is in a stable equilibrium position and there is a strength problem. If the force  $F$  reaches the buckling force  $F_K$  of the bar, the bar suddenly starts to buckle. The components, thus, lose their ability to function.

Buckling is usually a very sudden and abrupt process which causes large deformations.

## Different equilibrium positions

Stable equilibrium	Indifferent equilibrium	Unstable equilibrium
F-Force		

## Different equilibrium positions

Bars under pressure are a typical stability problem. Here, we investigate when a straight bar collapses. The critical buckling force  $F_K$  describes the smallest possible compressive force under which the bar buckles. The critical buckling stress  $\sigma_K$  is the stress that occurs at the critical buckling force  $F_K$ .

The buckling force for pressure-loaded bars depends on the support conditions, bending stiffness and geometry of the shape of the bar cross-section. Euler's four buckling cases are taken as the basis for the study of the bending stability of bars with constant bending stiffness.

## Basic knowledge

## BUCKLING AND STABILITY

## Euler's buckling cases

The mathematician and physicist Leonhard Euler defined four typical buckling cases to calculate the buckling force. For each of these cases, there is a buckling length coefficient  $\beta$  that is used to determine the buckling length  $L_K$ .

Pinned-Pinned	Fixed-Free	Fixed-Pinned	Fixed-Fixed
buckling length coefficient $\beta = 2$ buckling length $L_K = L \cdot \beta$	buckling length coefficient $\beta = 1$ buckling length $L_K = L \cdot \beta$	buckling length coefficient $\beta = 0.7$ buckling length $L_K = L \cdot \beta$	buckling length coefficient $\beta = 0.5$ buckling length $L_K = L \cdot \beta$

P force, L bar length,  $L_K$  buckling length,  $\beta$  buckling length coefficient

Determining the buckling force  $F_K$ 

$$F_K = \frac{\pi^2 \cdot E \cdot I}{L_K^2}$$

$F_K$  critical buckling force,  $L_K$  bar length,  $E$  elastic modulus  $I$  axial second moment of the cross-section area

Determining the buckling stress  $\sigma_K$ 

To determine the buckling stress we use the degree of slenderness  $\lambda$  as a material parameter and the moment of area radius  $i$ .

$$\lambda = \frac{\beta \cdot L}{i} \quad i = \sqrt{\frac{I}{A}} \quad \sigma_K = \frac{\pi^2 \cdot E}{\lambda^2}$$

$\sigma_K$  buckling stress,  $E$  elastic modulus,  $\lambda$  degree of slenderness,  $\beta$  buckling length coefficient,  $L$  bar length,  $i$  moment of area radius,  $A$  cross-section area of the buckled bar,  $I$  second moment of area